

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY AVERY

PROJECT DESCRIPTION US 19E AND NC 194 (ELK PARK HWY) IMPROVEMENTS

INVENTORY

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-5911	1	16

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOLI TEST DATA AVAILABLE MAY BE REVEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL LENGMEERING UNIT AT 1991 707-6860. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOLI TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU UNP-LACED TEST DATA CAN BE RELIED ON ONLY TO THE DECREE OF RELIABILITY INHERENT IN THE STANDARD TEST WETHO. THE OBSERVED WATER LEVELS OR SOLL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOLL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONTENS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NOTES:

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- TES: THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR SITEMISSION OF NO INFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE. 2.

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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

			SO	IL DE	SCRIPT	ION						GR	ADATION						ROCK DES	SCRIPTION
SOIL IS		UNCONSOL	DATED, SE	MI-CONSO	IDATED, OF	₹ WEATHERE	D EARTH MA	TERIALS TH	AT CAN	WELL GRADED - INDICAT	res a Gr	OOD REPRESEN	NTATION OF PARTIC	CLE SIZES FR	OM FINE TO COARSE.	HARD ROCK	IS NON-COAS	STAL PLAI	N MATERIAL THAT W	OULD YIELD SPT REFUSAL IF TESTED
ACCORDI	NG TO THE	STANDARD	PENETRAT	IN TEST	(AASHTO T	206, ASTM	D1586). SOI	L CLASSIFI	CATION	UNIFORMLY GRADED - IN GAR-GRADED - INDICATE	NDICATES	5 THAT SOIL	PARTICLES ARE AL	L APPROXIMA	TELY THE SAME SIZE.	SPT REFUSA	AL IS PENET	RATION BY	A SPLIT SPOON SA	AMPLER EQUAL TO OR LESS THAN 0.1 F
	BASED ON T	HE AASHTO	SYSTEM. P	ASIC DES	CRIPTIONS	GENERALLY	INCLUDE T	HE FOLLOWI	NG:				ITY OF CRAI			BLOWS IN N REPRESENTE	ION-COASTAL	L PLAIN M NE OF WEA	MATERIAL, THE TRA	NSITION BETWEEN SOIL AND ROCK IS
AS	S MINERALC	GICAL COMP	OSITION, A	NGULARIT	Y, STRUCTU	RE, PLASTIC	ITY, ETC. FO	R EXAMPLE.	•			HNOULHN		ESIGNATED BY	THE TERMS.	ROCK MATER	IALS ARE T	TYPICALLY	DIVIDED AS FOLLOW	/S:
۱ ۱	VERY STIFF.	GRAY.SILTY CL	AY, MOIST WI	TH INTERE	BEDDED FIN	E SAND LAYE	RS.HIGHLY PL	ASTIC.A-7-6		ANGULAR, SUBAN	NGULAR,	SUBROUNDED, (DR ROUNDED.	COTONATED DI	THE TERMO.	WEATHERED	y.	(S)(S)	NON-COASTAL PLAT	IN MATERIAL THAT WOULD YIELD SPT
051/504	5	OIL LE	JENU P	INU AP	<u>ISHIU</u>	LASSI		N			MI	NERALOGI	CAL COMPOSI	ITION		RUCK (WR)		5 5	100 BLOWS PER FU	JUI IF TESTED.
CLASS.		URANULAR MA (≤ 35% PASSI	IERIALS		(> 35% P/	ISSING #200	0	RGANIC MATER	IALS	MINERAL NAM	MES SUC	H AS QUARTZ	FELDSPAR, MICA, T	ALC, KAOLIN,	ETC.	CRYSTALLIN	E 🖉		WOULD YIELD SPT	REFUSAL IF TESTED. ROCK TYPE INCI
GROUP	A-1	A-3	A-2		A-4 A-5	A-6 A-7	A-1. A-2	A-4. A-5		ARE USED IN	N DESCR!	IPTIONS WHEN	THEY ARE CONSID	DERED OF SIG	NIFICANCE.	RULK (LR)	2	<u>C.Z.</u>	GNEISS, GABBRO, SC	HIST, ETC.
CLASS.	А-1-а А-1-ь	A-2-4	A-2-5 A-2	-6 A-2-7		A-7-5, A-7-6	A-3	A-6, A-7				COMPF	RESSIBILITY			NON-CRYSTA			SEDIMENTARY ROCK	<pre>< THAT WOULD YEILD SPT REFUSAL IF</pre>
SYMBOL	000000000000000000000000000000000000000			222						SLIG	HTLY CO		5	LL < 31	50				ROCK TYPE INCLUE	JES PHYLLITE, SLATE, SANDSTONE, ETC.
2 PASSING	000000000000000000000000000000000000000			<u> </u>	<u></u>					HIGHL	LY COMP	RESSIBLE	L	LL > 50	50	SEDIMENTAR	Y ROCK		SPT REFUSAL. ROC	K TYPE INCLUDES LIMESTONE, SANDST
*10 s	50 MX						GRANULAR	SILT-	MUCK,		P	ERCENTAC	GE OF MATER	RIAL		(CP)			SHELL BEDS, ETC.	
*40 3 *200	30 MX 50 MX	51 MN	25 MY 25		26 MN 26 MI		SOILS	SOILS	PEAT			GRANULAR	SILT - CLAY	07.150					WCHIT	
	13 11/ 23 11/	10 11/ 33 11/	33 Pix 33		JO PIN 30 PIN	, 30 Pill 30 Pill	" 			TRACE OF ORGANIC M	ATTER	2 - 3%	<u>3 - 5%</u>	TRACE	1 - 10%	FRESH	HAMMER IF	5H, CRYSTAL F CRYSTALI	_S BRIGHT,FEW JOIN _INE.	IS MAY SHOW SLIGHT STAINING, ROCK R
PASSING #40								C MITU		LITTLE ORGANIC MAT	TER	3 - 5%	5 - 12%	LITTLE	10 - 20%	VERY SLIGHT	ROCK GENE	ERALLY FR	ESH. JOINTS STAINED.	SOME JOINTS MAY SHOW THIN CLAY CO
LL	-	- 40 M)	41 MN 40	MX 41 MN	40 MX 41 MN	40 MX 41 MM		ILE OR		MODERATELY ORGANIC		5 - 10% > 10%	12 - 20%	SOME HIGHLY	20 - 35% 35% AND ABOVE	(V SLI.)	CRYSTALS	ON A BROM	EN SPECIMEN FACE	SHINE BRIGHTLY. ROCK RINGS UNDER HAM
	6 MX	NP 10 M			.0 MX 10 MX			ERATE	ORGANIC			GROU				1	OF A CRYS	STALLINE N	ATURE.	
GROUP INDEX	0	0	8	4 MX	8 MX 12 MX	16 MX NU M	X AMUL	inis u⊧ Ganic	SOILS							SLIGHT (SLIJ)	1 INCH. OP	ERALLY FRE	ESH, JOINTS STAINED	AND DISCOLORATION EXTENDS INTO ROCK
OF MAJOR	GRAVEL. AND	FINE	JILTY OR CL	YEY	SILTY	CLAYEY	MA	TTER			WATER	R LEVEL IN B	SORE HOLE IMMEDIA	AIELY AFIER	DRILLING		CRYSTALS	ARE DULL	AND DISCOLORED. CR	YSTALLINE ROCKS RING UNDER HAMMER
MATERIALS	SAND	SAND	RAVEL AND	SANU	SULS	SUILS					STATI	IC WATER LEV	EL AFTER 24	HOURS		MODERATE	SIGNIFICAN	NT PORTION	IS OF ROCK SHOW DIS	SCOLORATION AND WEATHERING EFFECTS.
GEN. RATING		EXCELLENT T	0.000		FAIR		FAIR TO	POOR		<u> ∑Pw</u>	PERCH	HED WATER, SA	ATURATED ZONE, OR	WATER BEAR	ING STRATA	(MOD.)		I ROCKS, MO	IST FELDSPARS ARE D	JULL AND DISCOLORED, SOME SHOW CLAY.
AS SUBGRADE							POOR		GINDEL		SPRIN	NG OR SEEP					WITH FRES	SH ROCK.		
		PI OF A-7-5	UBGROUP IS	≤ LL - 34	J : PI OF A-7	-6 SUBGROUP I	IS > LL - 30			0 00				21.0		MODERATELY	ALL ROCK	EXCEPT O	JARTZ DISCOLORED OF	R STAINED. IN GRANITOID ROCKS.ALL FE
			<u>UNSISI</u>	ENCY	UK DE	NSENES:	5				<u> </u>	1ISCELLAI	NEUUS SYMBU	JLS		SEVERE (MOD. SEV.)	AND DISCO AND CAN F	JLORED AND BE EXCAVA) A MAJORITY SHOW H TED WITH A GEOLOGIS	<pre><aolinization. los<br="" rock="" severe="" shows="">ST'S PICK. ROCK GIVES "CLUNK" SOUND WE</aolinization.></pre>
PRIMARY S	OIL TYPE	COMP	CTNESS C	JR F	PANGE OF PENETRATIO	STANDARD	E COM	IGE OF UNC PRESSIVE S	ONFINED TRENGTH	ROADWAY EMB	ANKMEN'	T (RE) 25/02	5 DIP & DIP DIR	RECTION			<u>IF TESTEC</u>	D. WOULD YI	ELD SPT REFUSAL	
			515 FENCT		(N-V	ALUE)		(TONS/F1	r ²)	WITH SOIL DE	SCRIPTI	ON H	OF ROCK STRU	ICTURES		SEVERE	ALL ROCK	EXCEPT O	JARTZ DISCOLORED OF	R STAINED. ROCK FABRIC CLEAR AND EV
GENERAL	LY	VEF	Y LOOSE			: 4 10, 10				SOIL SYMBOL			DPT DMT TEST BOR	RING	SLOPE INDICATOR	(SEV.)	REDUCED I	IN STRENGT	H TO STRONG SOIL.	IN GRANITOID ROCKS ALL FELDSPARS AR
GRANULA	AR	MED	UUSE		10	TO 30		N/A					VST PMT				IF TESTED	D. WOULD Y	ELD SPT N VALUES >	> 100 BPF
(NON-CO	HESIVE)		DENSE		30	TO 50				THAN ROADWA	Y EMBAN		AUGER BORING	٩	TEST	VERY	ALL ROCK	EXCEPT O	JARTZ DISCOLORED OF	R STAINED. ROCK FABRIC ELEMENTS ARE
		VE	Y DENSE		`	50							CODE DODING			SEVERE	BUT MASS REMAINING	IS EFFECT	TIVELY REDUCED TO S	JOIL STATUS, WITH ONLY FRAGMENTS OF
GENERAL	LY	VE	RY SUFI SOFT		2	. 2 TO 4		< 0.25 0.25 TO	0.5	INFERRED SUL	L BUUNL		- CURE BURING		SUUNDING RUD		VESTIGES	OF ORIGIN	AL ROCK FABRIC REM	AIN. IF TESTED, WOULD YIELD SPT N VA
SILT-CL	AY	MED	UM STIFF		4	TO 8		Ø.5 TO 1	.0	INFERRED ROOM	CK LINE	MWO	MONITORING WE	ELL 🕂	TEST BORING WITH CORE	COMPLETE	ROCK REDL	UCED TO SI	DIL. ROCK FABRIC NO	T DISCERNIBLE, OR DISCERNIBLE ONLY IN
MATERIA (COHESI)	NL VF)	VE	STIFF		8 1	:015 T030		1 TO 2 2 TO 4	4				PIEZOMETER	$\overline{\frown}$	- SPT N-VALUE		SCATTERED	D CONCENTR	RATIONS. QUARTZ MAY	BE PRESENT AS DIKES OR STRINGERS.
			HARD		>	30		> 4	-		2 00000		' INSTALLATION	\bigcirc	SIT IN VIECE				POCK H	
			TEXTL	JRE OF	IAND V	N SIZE					R	ECOMMEN	DATION SYMB	OLS						PR DICK PREAKING OF HAND CRECIMENC
U.S. STD. SIE	EVE SIZE		4	10	40	60 20	0 270					LASSIFIED E	CAVATION -		IFIED EXCAVATION -	VENT HHRD	SEVERAL F	HARD BLOW	S OF THE GEOLOGIST	'S PICK.
OPENING (MN	4)		4.76	2.00	0.42	0.25 0.0	75 0.0 53					SUITABLE WAS		ACCEPTA	BLE, BUT NOT TO BE THE TOP 3 FEET OF	HARD	CAN BE SC	CRATCHED (BY KNIFE OR PICK ON	NLY WITH DIFFICULTY. HARD HAMMER BLO
BOULDER	R CC	DBBLE	GRAVEL		COARSE	FIN	IE I	SILT	CLAY			CEPTABLE DEG	RADABLE ROCK	EMBANK	MENT OR BACKFILL		TO DETACH	h hand spe	ECIMEN.	
(BLDR.)		COB.)	(GR.)	(CSE. SD.)	(F f	ND SD.)	(SL.)	(CL.)			ABBR	EVIATIONS			HODERATELY	CAN BE SC	CRATCHED E	BY KNIFE OR PICK. G	JUGES OR GROOVES TO 0.25 INCHES DEE ST'S PICK HAND SPECIMENS CAN BE DE
GRAIN MM	305	75		2.0		0.25	0.05	0.005	j	AR - AUGER REFUSAL		MED	MEDIUM	VST -	VANE SHEAR TEST		BY MODER	ATE BLOWS	•	
SIZE IN.	12	3								BT - BORING TERMINATED	G	MICA	MICACEOUS	WEA	WEATHERED	MEDIUM	CAN BE GF	ROOVED OR	GOUGED 0.05 INCHES	DEEP BY FIRM PRESSURE OF KNIFE OR
	9	SOIL MO	ISTUR	<u>- cc</u>	RRELA	TION OF	TERMS	5		CL CLAY	N TEST	MOD	MODERATELY	γ-ι γ-ι	NIT WEIGHT	HARD	CAN BE EX	XCAVATED I	IN SMALL CHIPS TO F	'EICES I INCH MAXIMUM SIZE BY HARD B
SOIL	MOISTURE	SCALE	FIF	LD MOIS	TURE					CSE COARSE	N IESI	ORG	ORGANIC	7d -	ALL ONLY WEIGHT	SOFT	CAN BE GE	ROVED OR (Gouged Readily by F	KNIEF OR PICK. CAN BE EXCAVATED IN F
(ATT	ERBERG LI	(MITS)	C	ESCRIPT	ON		FIELD MO	ISTURE DES	SCRIFTION	DMT - DILATOMETER TES	T T	PMT -	PRESSUREMETER TE	EST <u>SAN</u>	IPLE ABBREVIATIONS		FROM CHIF	PS TO SEVE	RAL INCHES IN SIZE	BY MODERATE BLOWS OF A PICK POINT.
			- 5	SATURATE	.D -	USUALLY I	IQUID; VER	Y WET, USU	ALLY	e - VOID RATIO	TION TE:	SD S	AND, SANDY	S - B	SPLIT SPOON		PIECES CA	AN BE BROK	EN BY FINGER PRESS	URE.
				(SAT.)		FROM BEL	OW THE GR	OUND WATE	R TABLE	F - FINE		SL S	ILT, SILTY	ST - 1	SHELBY TUBE	SOFT	OR MORE !	ARVED WITH	I KNIFE. CAN BE EXC SS CAN BE BROKEN E	AVATED READILY WITH POINT OF PICK. P BY FINGER PRESSURE, CAN BE SCRATCHE
		LIMII								FOSS FOSSILIFEROUS	TUPES	SLI S	BLIGHTLY	RS - I	ROCK	00.1	FINGERNAI	IL.		
RANGE <			-)	VET - (W)		ATTAIN OF	TIMUM MOI	STURE	J	FRAGS FRAGMENTS	TONES	w - MC	DISTURE CONTENT	CBR -	CALIFORNIA BEARING		FRACTUF	RE SPA	CING	BEDDING
(PI) PL L		IC LIMIT								HI HIGHLY		V - VEI	RY		RATIO	TERM			SPACING	TERM T
			- 1	10IST - /	(M)	SOLID: AT	OR NEAR C	РТІМИМ МС	ISTURE	EO	<u>UIPME</u>	NT USED	ON SUBJECT	<u>T_PROJEC</u>	Т	VERY WII	JE	MORE	THAN 10 FEET	VERY THICKLY BEDDED
OM		UM MOISTUR	E			002104	0			DRILL UNITS:	ADVAN	NCING TOOLS:		HAMMER T	YPE:	MODERAT	ELY CLOSE	1	TO 3 FEET	THINLY BEDDED 0.16
	T					REQUIRES		WATER TO	ı	CME-45C		CLAY BITS			DMATIC MANUAL	CLOSE	065	0.16	S TO 1 FOOT	VERY THINLY BEDDED 0.03
			- [JRY - (D)		ATTAIN OF	TIMUM MOI	STURE	5			6" CONTINUOUS	FLIGHT AUGER	CORE SIZE			JE	LESS	THHN 0.10 FEET	THINLY LAMINATED <
-	1				TICITY							8" HOLLOW AUG	GERS	П-в	-н				INDUF	ATION
						(PI)	-		:т u	CME-550		HARD FACED F	INGER BITS			FOR SEDIME	NTARY ROCK	KS. INDURAT	ION IS THE HARDEN	ING OF MATERIAL BY CEMENTING, HEA
NON	PLASTIC		!	CHSTILL	0-5	<u>vr 17</u>	L	VERY LOW	/			TUNGCARBIDE	INSERTS			ERIA	RI F		RUBBING WITH	FINGER FREES NUMEROUS GRAINS;
SLIC	HTLY PLA	STIC			6-15			SLIGHT		VANE SHEAR TEST				HAND TOO	LS:	r NIH			GENTLE BLOW	BY HAMMER DISINTEGRATES SAMPLE.
MODI HTGH	ERATELY P	'LASTIC IC		1 26 1	6-25 OR MORF			MEDIUM HIGH					NOTES TEET	POS	F HOLE DIGGER	MODE	RATELY IND	JURATED	GRAINS CAN BE	SEPARATED FROM SAMPLE WITH STE
										PURTABLE HOIST		IRILUNE	SIEEL IEETH	X HAN) AUGER				BREAKS EASILY	WHEN HIL WITH HAMMER.
					LON					1 🗆		IRICONE	TUNGCARB.	Soul	NDING ROD	INDU	RATED		GRAINS ARE DI	FFILULI IU SEPARATE WITH STEEL PI BREAK WITH HAMMER.
DESCRIPT	IONS MAY	INCLUDE C	JLOR OR (COLOR CC	MBINATION	S (TAN, REI	D. YELLOW-	BROWN, BLU	E-GRAY).			CORE BIT			SHEAR TEST				SHARP HAMMED	
MO	UIFIERS SI	UCH AS LIC	нí,DARK,	STREAKE	J, ETC. ARE	. USED TO	DESCRIBE	APPEARANCE	- .					$ \square _$		EXTR	EMELY INDUF	RATED	SAMPLE BREAK	S ACROSS GRAINS.





AN INFERRED	TERMS AND DEFINITIONS
PT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
OFTEN	ARENACEDIS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
N VALUES >	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
THAT UDES GRANITE,	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
PLAIN TESTED.	<u>CALCAREOUS (CALC.)</u> - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. <u>COLLUVIUM</u> - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
AY NOT YIELD DNE,CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
NGS UNDER	<u>DIKE</u> - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
TINGS IF OPEN	<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
MER BLOWS IF	<u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
UP TO FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
in Rock has	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
S COMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
DSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
EN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
DENT BUT KAOLINIZED	ITS LATERAL EXTENT.
	LENS - A BODT OF SUIL OF ROLK THAT THINS OUT IN ONE OF MORE DIRECTIONS.
	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
NLY MINOR UES < 100 BPF	OF AN INTERVENING IMPERVIOUS STRATUM.
SMALL AND	RESIDUAL (RES.) SUIL - SUIL FURMED IN PLACE BY THE WEATHERING OF RUCK.
SAPROLITE IS	ROCK SECHENTS EQUAL TO OR OREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
REQUIRES	$\underline{SAPROLITE\ (SAP.)}$ - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
WS REQUIRED	<u>SILL</u> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THING COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
CAN BE	<u>SLICKENSIDE</u> - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
PICK POINT. OWS OF THE	STANDARD PENETRATION TEST (PENETRATION RESISTANCE)(SPT) - NUMBER OF BLOWS (N OR BPF)OF A 140 LB.HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
RAGMENTS SMALL, THIN	<u>STRATA CORE RECOVERY (SREC.)</u> - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
IECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
	BENCH MARK: N/A
IICKNESS 4 FEET	
- 4 FEET	ELEVATION: FEET
- 1.5 FEET - 0.16 FEET	NOTES:
- 0.03 FEET	ROADWAY DESIGN FILES, PROVIDED BY VAUGHN & MELTON.
BUG FEEI	NORTHING AND FASTINGS OBTAINED LISING A TRIMPLE CEOTY ROPING
, PRESSURE, ETC.	ELEVATIONS FOR ROADWAY BORINGS WERE OBTAINED USING PROVIDED
L PROBE:	FIAD = FILLED IN AFTER DRILLING
·	
OBE:	



STATE	STATE	PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.		R-5911	3	16
STATE PRO	J. NO.	F. A. PROJ. NO.	DESCRIPT	TON
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ECS Southeast, LLP

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August 27, 2021

WBS NO:	N/A
TIP NO:	R-5911
COUNTY:	Avery
DESCRIPTION:	US 19E and NC 194 (Elk Park Hwy) Improvements

SUBJECT: **Geotechnical Report – Inventory**

Project Description

The project consists of widening US 19E to include a new turning lane along its approach to the intersection of US 19E and NC 194 (Elk Park Hwy) in Avery County, North Carolina. Improvements include widening and resurfacing US 19E (-L-) and resurfacing SR 1169/Substation Road (-Y2-). US 19E will be widened along its approach to NC 194 (Elk Park Hwy) to accommodate a new 12-foot wide turning lane with a taper. The project is approximately 1,040 linear feet in length, measured along the US 19E (-L-) alignment.

At the beginning of the project, the proposed road grade along the -L- alignment centerline begins at approximate elevation 3,093 feet and slopes downward to a low elevation of approximately 3,080 feet in at the end of the project near Station 20+39. Soils encountered in the proposed construction areas generally consisted of roadway embankment underlain by alluvial and residual soils. Mass grading will generally be limited to the widened areas with cut depths on the order of approximately 2 feet or less and fill heights on the order of 4 feet or less.

The following roads are included as part of this exploration:

<u>Line</u>	Road Name	<u>Station (±)</u>	<u>Offsets</u>
-L-	US 19E	10+00 to 20+39	LT to RT
-Y2-	SR 1169/Substation Road	11+75 to 12+41	LT to RT

A geotechnical field investigation was performed by ECS in July 2021. During this time period, a total of six (6) hand auger borings supplemented with Dynamic Cone Penetrometer (DCP) tests were performed along the -L- alignment within the proposed widened areas. Representative soil samples were collected for visual classification in the field and for analysis by ECS's testing laboratory.

Physiography and Geology

In accordance with the Geologic Map of North Carolina, 1985, the project corridor is located in the Blue Ridge Physiographic Province of North Carolina. The eastern Blue Ridge consists of a variety of igneous and high-grade metamorphic rocks, including numerous granite bodies. The metamorphic rocks were originally part of the Precambrian basement, as were the volcanic and sedimentary rock laid down as the proto-Atlantic Ocean began to close during the early Paleozoic era. Migmatites, rocks with both metamorphic and igneous characteristics, make up other eastern Blue Ridge rocks and reveal the very high temperatures reached by these rocks during mountain uplift. The soils in the Blue Ridge Province typically consist of residuum (weathered in-place soils) derived from the parent bedrock, alluvium in the valleys, and sometimes colluvium draping the hillsides.

Soil Properties

Soils within the area of this project have been divided into three categories: roadway embankment, alluvial, and residual soils.

Roadway Embankment: Roadway Embankment (R.E.) soils generally consist medium stiff to stiff fine to coarse sandy silt (A-4) with trace amounts of organics and gravel. The roadway embankment extends to depths ranging from approximately 1.5 to 2.0 feet below existing grades. Laboratory testing of the roadway embankment soils indicated Plasticity Indices (Pl's) ranging from 5 to 6 for the silty (A-4) soils.

Alluvial Soils: Alluvial soils generally consist of soft fine to coarse sandy clay (A-6) and soft to stiff fine to coarse sandy silt (A-4). Laboratory testing of the alluvial soils indicated a Plasticity Index (PI) of 11 for the clayey (A-6) soil.

Residual Soils: Residual soils throughout the project corridor are derived from the weathering of the underlying parent bedrock. A majority of the residual soils encountered generally consisted of medium stiff to stiff fine to coarse sandy silt (A-4), medium stiff clayey silt (A-5), and medium stiff fine to coarse sandy clay (A-6). Laboratory testing of the residual soils indicated a Plasticity Index (PI) of 9 for the silty (A-4) soils, a PI of 13 for the clayey (A-6) soils.

Groundwater Properties

Groundwater levels were measured in the borings both immediately after drilling (0-hr reading). At the time of drilling, water was encountered in 2 borings at depths ranging from approximately 3.3 to 5.5 feet below existing grades, which corresponds to elevations ranging from 3,078.6 feet to 3,073.0 feet. The remaining borings were dry when groundwater readings were taken. Due to the proximity to the existing roadway, the borings were filled in after augering, making stabilized (24-hr) water readings unobtainable. The recovered soil samples were generally described as moist above the groundwater level and moist to saturated below the groundwater level

Areas of Special Geotechnical Interest

1) <u>Soft/Very Loose Soils</u>: The following areas contain relatively soft or very loose soils that have the potential for subgrade instability, embankment stability or long-term settlement problems during construction:

<u>Line</u>	Station (±)	<u>Offsets</u>
-L-	16+75 to 18+75	LT
-Y2-	11+75 to 12+41	RT

2) <u>High Groundwater</u>: Groundwater was encountered within six feet of the proposed subgrade at the following locations. This has the potential to cause subgrade instability and/or constructability issues.

<u>Line</u>	<u>Station (±)</u>	<u>Offsets</u>
-L-	12+75 to 14+75	LT
-L-	16+75 to 18+75	LT

3) <u>Hand Auger Refusal</u>: The following areas encountered hand auger refusal above or within 6 feet of proposed subgrade. Hand auger refusal could indicate areas of rocky embankment fill, hard or dense residual soils layers, weathered rock or crystalline rock.

<u>Line</u>	<u>Station (±)</u>	<u>Offsets</u>
-L-	10+75 to 12+75	LT
-L-	14+75 to 16+75	LT
-L-	18+75 to 20+39	LT

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L_{1000}	S-24	24' LT	10+09	3.0–4.0'	A-4(3)	38	9	29.2	16.4	32.3	22.1	87.6	68.2	51.2	24.9	_
L_{1400}	S-12	23' LT	14 + 03	1.0–2.0'	A-6(8)	40	13	21.6	14.0	28.4	36.0	97.2	82.6	65.7	26.8	_
L_{1600}	S-9	27' LT	16+01	0.0–1.0'	A-4(0)	30	6	31.2	21.0	22.0	25.8	82.7	63.0	44.2	24.7	_
L_1800	S-5	23' LT	18+03	2.0-3.0'	A-6(5)	36	11	22.0	16.4	31.8	29.8	92.6	78.8	60.6	26.9	_
L_{2000}	CBR-1	52' LT	19 + 95	0.0-2.0'	$A-\overline{4(0)}$	33	5	44.6	14.9	33.0	7.5	89.5	58.0	39.2	22.8	_

LAB TECHNICIAN: C. RUPERT

NCDOT CERTIFICATION NO. 112-01-1003

PROJECT REFERENCE NO. R-5911





